General Notes

1.1 DESIGN LOADS

A. THE STRUCTURE WAS DESIGNED FOR THE LIVE LOADS SHOWN BELOW AND DEAD LOADS AS REQUIRED BY CONSTRUCTION IN ACCORDANCE WITH I.B.C.2015. LOADS DUE TO SNOW LOAD BUILD-UP WERE CONSIDERED IN DESIGN OF STRUCTURAL COMPONENTS ADJACENT TO PARAPETS, HIGH BUILDING WALLS, ETC. INCREASE IN THESE LOADINGS, DUE TO CHANGE IN FUNCTION, CONSTRUCTION MATERIALS, ETC., TO HAVE WRITTEN APPROVAL FROM THE DESIGNING STRUCTURAL ENGINEER.

B. THE BASIC STABILITY OF THE STRUCTURE IS DEPENDENT UPON THE DIAPHRAGM ACTION OF FLOORS, WALLS, AND ROOF ACTING TOGETHER. PROVIDE GUYS, BRACES, STRUTS, ETC., TO ACCOMMODATE LIVE, DEAD, AND WIND LOADS UNTIL FINAL CONNECTIONS BETWEEN THESE ELEMENTS ARE MADE.

C. MECHANICAL UNITS WITH WEIGHTS SHOWN IN PLAN AND SUPPORTED BY THE STRUCTURE WERE CONSIDERED IN THE DESIGN OF THE STRUCTURE. ADDITIONAL MECHANICAL EQUIPMENT NOT SHOWN ON STRUCTURAL DRAWINGS AND HAVING A WEIGHT IN EXCESS OF 400 POUNDS SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.

D. LIVE LOADS SHOWN BELOW ARE IN POUNDS PER SQUARE FOOT (PSF).
ROOF LIVE LOAD: 30 GROUND SNOW LOAD (PG): 30
FLOOR LIVE LOAD: 100 FLAT ROOF SNOW LOAD(PF):21
SNOW LOAD IMPORTANCE FACTOR: 1.0 SNOW EXPOSURE FACTOR (Ce): 1.0

E. WIND CRITERIA:
BASIC WIND SPEED: 90 MPH (3 SECOND GUST)
WIND IMPORTANCE FACTOR (Iw): 1.0 , OCCUPANCY CATEGORY: II
MAIN WIND FORCE SYSTEM EXPOSURE CATEGORY: B
INTERNAL PRESSURE COEFFICIENT: +/- 0.18
HORIZONTAL PRESSURES VERTICAL PRESSURES
WALLS:+10.5 & 15.8 ROOF: -8

COMPONENTS & CLADDING:
ROOF ZONE 1: 10 & -16.3 WALL ZONE 4:14.1 & -15.4 PARAPET: 4.2
ROOF ZONES 2&3: 13 & -19.3 WALL ZONE 5: 14.1 & -17.3

F. SEISMIC CRITERIA:
Ss = 12.4 Fa = 1.6 Sds = 0.132
S1 = 5.1 Fv = 2.4 Sd1 = 0.082
SEISMIC SITE CLASSIFICATION: D
SEISMIC OCCUPANCY CATEGORY: M, IMPORTANCE FACTOR: 1.0
BASIC SEISMIC-FORCE-RESISTING SYSTEM:LIGHT FRAMED WALL SYSTEM WITH FLAT STRAP BRACING , (R):4
SEISMIC BASE SHEAR (V): 5.6 KIPS
SEISMIC DESIGN CATEGORY: B, RESPONSE COEFFICIENT(Cs): 0.034
ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCEANALYSIS

1.5 SUBMITTALS

A. BEFORE SUBMISSION OF SHOP DRAWINGS, CONTRACTOR SHALL HAVE DETERMINED AND VERIFIED QUANTITIES, DIMENSIONS, SPECIFIED PERFORMANCE CRITERIA, INSTALLATION REQUIREMENTS, MATERIALS, CATALOG NUMBERS, AND SIMILAR DATA WITH RESPECT THERETO AND REVIEWED OR COORDINATED EACH SHOP DRAWING WITH OTHER SHOP DRAWINGS AND SAMPLES AND WITH THE REQUIREMENTS OF THE WORK AND THE CONTRACT DOCUMENTS.

B. AFTER CHECKING AND VERIFYING COMPLIANCE WITH CONTRACT DOCUMENTS AND ACTUAL FIELD CONDITIONS, CONTRACTOR SHALL SUBMIT, FOR REVIEW, SHOP DRAWINGS REFERENCED IN THE INDIVIDUAL MATERIALS SECTIONS. CONTRACTOR SHALL STAMP OR PROVIDE A SIMILAR WRITTEN INDICATION THAT CONTRACTOR HAS REVIEWED THE SUBMISSION AND IS SATISFIED THAT MATERIALS SHOWN ARE IN COMPLIANCE WITH THE CONTRACT DOCUMENTS.

C. A REVIEW PERIOD OF 5 WORKING DAYS WILL BE REQUIRED FOR SHOP DRAWING REVIEW, OF EACH UNIT TYPE. SHOP DRAWING SUBMISSION OF MULTIPLE COMPONENT TYPES WILL REQUIRE ADDITIONAL REVIEW TIME. SHOP DRAWINGS WILL BE FORWARDED TO ARCHITECT OR CLIENT FOR THEIR REVIEW BEFORE RETURNING TO THE CONTRACTOR.

2.3 FOUNDATIONS

A. A SOIL BEARING CAPACITY OF 2000 PSF WAS USED FOR FOOTING DESIGN. ENGAGE THE SERVICES OF A GEOTECHNICAL ENGINEER TO VERIFY EXCAVATIONS AND SOIL BEARING CAPACITY. IF SOIL OF THIS CAPACITY IS NOT ENCOUNTERED AT ELEVATIONS INDICATED, CONTACT ENGINEER OF RECORD (EOR).

B. INSTALL FOOTING BOTTOMS 1'-0" MINIMUM BELOW EXISTING GRADE OR COMPACTED FILL, WHICHEVER IS HIGHER.

C. INSTALL EXTERIOR FOOTING BOTTOMS 2'-6" MINIMUM BELOW FINISH GRADE.

D. BASEMENT AND FOUNDATION WALLS ARE DEPENDENT UPON THE COMPLETED INSTALLATION OF FLOORS AND ROOFS FOR THEIR STABILITY. DO NOT PLACE BACKFILL UNTIL THESE ELEMENTS ARE COMPLETELY INSTALLED, OR PROVIDE SHORING AND BRACING.

E. COMPACT FILL AND BACKFILL TO 95% OF ASTM D-698 OR AS DETERMINED BY THE GEOTECHNICAL ENGINEER. PERFORM FILL AND BACKFILL OPERATIONS UNDER THE DIRECT SUPERVISION OF THE GEOTECHNICAL ENGINEER.

F. PRIOR TO POURING CONCRETE, ENGAGE THE SERVICES OF A PROFESSIONAL GEOTECHNICAL ENGINEER (REGISTERED IN THE JURISDICTION WHERE THE PROJECT IS LOCATED), TO PERFORM TESTS, BORINGS, ETC., REQUIRED TO CERTIFY THAT THE SOIL BEARING CAPACITY MEETS OR EXCEEDS THAT SHOWN IN THE GENERAL NOTES ABOVE. GEOTECHNICAL ENGINEER SHALL VERIFY SUBGRADE CAPACITIES PRIOR TO INSTALLATION OF DRAINAGE FILL AND MOISTURE BARRIER.

3.1 CONCRETE

A. UNLESS GOVERNED BY BUILDING CODE OR LOCAL AMENDMENTS: CONCRETE WORK INCLUDING FORMING, MIXING, PLACING AND CURING SHALL BE IN ACCORDANCE WITH ACI 301. PLACEMENT OF REINFORCING SHALL BE IN ACCORDANCE WITH ACI 315 and 318. WHEN THERE IS A CONFLICT, THE MOST STRINGENT IS TO APPLY.

B. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION OR ERECTION. REPRINTS OF CONTRACT DRAWINGS ARE NOT ACCEPTABLE. SUBMIT DESIGN MIXES FOR EACH CLASS OF CONCRETE PRIOR TO USE.

C. CONCRETE REINFORCING: ASTM A-615, GRADE 60.

D. WELDED WIRE REINFORCEMENT: ASTM A-185.

E. PORTLAND CEMENT: ASTM C-150, TYPE I.

F. BLENDED HYDRAULIC CEMENT: ASTM C-595.

G. AGGREGATE: ASTM C-33. 1" MAXIMUM FOR FOOTINGS, WALLS AND SLABS ON GRADE, 2" MAXIMUM FOR THIN SLABS AND 3/8" FOR WALL FILL.

H. CONCRETE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF: 3,000 PSI.

I. EXTERIOR CONCRETE TO BE AIR-ENTRAINED AND SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF: 3,500 PSI.

J. WATER CEMENT RATIO NOT TO EXCEED 0.54 FOR 3,000 PSI CONCRETE AND 0.45 FOR AIR ENTRAINED CONCRETE.

K. INSTALL WELDED WIRE REINFORCEMENT 2" BELOW UPPER SURFACE OF CONCRETE SLAB.

L. REINFORCING FOR FOOTINGS AND OTHER CONCRETE USING EARTH FORMS SHALL HAVE 3" CONCRETE COVER. REINFORCING FOR CONCRETE EXPOSED TO GROUND OR WEATHER AFTER REMOVAL OF FORMS SHALL HAVE 2" CONCRETE COVER. REINFORCING SHALL HAVE 3/4" CONCRETE COVER FOR SLABS AND WALLS AND 1 1/2" COVER FOR BEAMS, GIRDERS, AND COLUMNS.

M. LAP CONTINUOUS FOOTING REINFORCING 44 BAR DIAMETERS AT SPLICES.

N. USE A WATER REDUCING ADMIXTURE IN ALL CONCRETE.

O. USE A MINIMUM OF 5 1/2 BAGS OF CEMENT AND A MAXIMUM OF 6 1/2 GALLONS OF WATER PER BAG FOR EACH CUBIC YARD OF CONCRETE.

P. SLUMP - AS REQUIRED BY ACI (211.1), EXCEPT THAT SLABS-ON-GRADE AND THIN-FRAMED SLABS SHALL HAVE A MAXIMUM SLUMP OF 4". SHOULD EXTRA WATER BE REQUIRED BEFORE DEPOSITING CONCRETE AND WATER/CEMENT RATIO OF ACCEPTED MIX DESIGN HAS NOT BEEN EXCEEDED, GENERAL CONTRACTOR'S SUPERINTENDENT SHALL HAVE SOLE AUTHORITY TO AUTHORIZE ADDITION OF WATER. ANY ADDITIONAL WATER ADDED TO MIX AFTER LEAVING BATCH PLANT SHALL BE INDICATED ON THE TRUCK TICKET AND SIGNED BY PERSON RESPONSIBLE. SUBMIT COPY OF TRUCK TICKET FOR REVIEW.

Q. AIR ENTRAIN EXTERIOR EXPOSED CONCRETE 5% +/- 1%.

R. NO CALCIUM CHLORIDE WILL BE PERMITTED IN CONCRETE.

S. ENGAGE THE SERVICES OF A TESTING AGENCY APPROVED BY THE ARCHITECT TO PERFORM TESTS OF CONCRETE. TAKE A MINIMUM OF 5 CYLINDERS FOR EACH CLASS OF CONCRETE POURED IN ANY ONE DAY. PERFORM 1 SLUMP TEST PER TRUCK LOAD OF CONCRETE.

T. PROVIDE TWO COMPRESSION TESTS AT 7 DAYS, TWO AT 28 DAYS, AND RETAIN ONE TEST FOR ADDITIONAL TESTING AS REQUIRED. COMPRESSIVE STRENGTH OF CONCRETE AT 7 DAYS TO ACHIEVE AT LEAST 65% OF MINIMUM DESIGN STRENGTH.

U. SUBMIT CAPACITIES OF ANCHORS AND POWER ACTUATE FASTENERS FOR REVIEW PRIOR TO USE.

4.1 MASONRY

A. UNLESS GOVERNED BY BUILDING CODE OR LOCAL AMENDMENTS: MANUFACTURE AND INSTALL MASONRY IN ACCORDANCE WITH (ACI 530/ASCE 5/TMS 402),(ACI 530.1/ASCE 6/TMS 602). WHEN THERE IS A CONFLICT, THE MOST STRINGENT IS TO APPLY.

B. BLOCK: CONCRETE MASONRY UNITS: 1,900 PSI COMPRESSIVE STRENGTH (AVERAGE OF THREE UNITS). ASTM C-90 WITH MINIMUM DENSITY OF 125 LBS. PER CU. FT. FOR NORMAL WEIGHT AND 100 LBS. PER CU. FT. FOR LIGHT WEIGHT UNITS.

C. DESIGNED ? m: 1,500 PSI. AT 28 DAYS.

D. BLOCK USED IN EXTERIOR WALLS, INTERIOR BEARING WALLS, AND WALLS WITH VERTICAL STEEL REINFORCING SHALL BE MANUFACTURED AND LAID SUCH THAT WEBS ARE IN COMPLETE ALIGNMENT.

E. MORTAR: ASTM C-270 TYPE S. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS TO BE 1,800 PSI.

F. GROUT FOR WALL FILL: ASTM C-476, 2000 PSI MINIMUM AT 28 DAYS WITH 65% OF STRENGTH AT 7 DAYS. USE FINE AGGREGATE SIZE #1 IN ACCORDANCE WITH ASTM C-404. MIX TO A SLUMP OF 8 TO 11 INCHES UTILIZING WATER REDUCING ADMIXTURES. FLY ASH AND BLAST-FURNACE SLAG (UP TO 25%) MAY BE USED. HOWEVER, FLY ASH AND BLAST-FURNACE SLAG CAN PRODUCE SLOWER INITIAL STRENGTH GAIN, WHICH NEEDS TO BE CONSIDERED IN COLD WEATHER.

G. GROUT FOR BOLT EMBEDS AND UNDER BEAM OR BASE PLATES: ASTM C-1107, 5000 PSI, NON-SHRINK.

H. REINFORCING: ASTM A-615, GRADE 60.

I. SINGLE WYTH 6" THICK OR MORE CONSTRUCTED WITH BRICK, BLOCK, OR ANY COMBINATION THEREOF (EXCEPT 4" VENEERS SEPARATED BY AIR SPACE) SHALL HAVE GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCING AT 16" O/C MAXIMUM WITH PREFABRICATED CORNER AND "T" PIECES UNLESS NOTED. PARAPET WALLS SHALL HAVE HORIZONTAL JOINT REINFORCING AT 8" O/C. LAP SPLICES 6" MIN. PROVIDE AN ADDITIONAL ROW ABOVE AND BELOW OPENINGS AND EXTEND 2'-0" BEYOND JAMBS. STOP HORIZONTAL JOINT REINFORCING EACH SIDE OF CONTROL AND EXPANSION JOINTS.

J. HORIZONTAL JOINT REINFORCING SHALL BE IN ACCORDANCE WITH ASTM A-951, SHALL BE MANUFACTURED FROM 9 GAGE (0.148) MIN. COLD DRAWN STEEL WIRE CONFORMING TO ASTM A-82, AND SHALL CONSIST OF TWO DEFORMED LONGITUDINAL SIDE RODS WELDED AT 16" PLUS OR MINUS INTERVALS TO A PERPENDICULAR CROSS ROD FORMING A LADDER DESIGN. CROSS ROD AND SIDE RODS SHALL BE LOCATED IN THE SAME PLANE AS THE LONGITUDINAL RODS. OUT-TO-OUT SPACING OF SIDE RODS SHALL BE APPROXIMATELY 2" LESS THAN THE NOMINAL WALL THICKNESS.

K. JOINT REINFORCEMENT IN EXTERIOR WALLS TO BE HOT DIPPED GALVANIZED, AFTER FABRICATION, IN ACCORDANCE WITH ASTM A-153, CLASS B2 (1.80 OZ./SQ. FT.).

L. FILL CELLS OF BLOCK SOLID WITH MORTAR IN COURSE DIRECTLY BELOW CHANGES IN THICKNESS AND BOND.

M. WALLS NOTED AS FILLED SOLID AND CELLS CONTAINING VERTICAL REINFORCING SHALL HAVE CORES OF BLOCK FILLED WITH GROUT IN SIX COURSE MAXIMUM LIFTS.

N. MASONRY WALLS SHALL BE CONNECTED TO STEEL COLUMNS AND BEAMS WITH 16 GAGE GALVANIZED WELD ON CHANNEL SLOTS 8" LONG X 1 1/2" WIDE SPACED AT 16" O/C (UNLESS ALTERNATE CONNECTIONS ARE OUTLINED IN SECTIONS). WELD TO BEAM WITH TWO 1/16" FILLET WELDS 1" MINIMUM LONG EACH SIDE OF SLOT (4 TOTAL EACH SLOT). CHANNEL SLOT ANCHORS SHALL BE 3/16" DIAMETER WIRE WITH GALVANIZED COATING IN COMPLIANCE WITH ASTM A-153, CLASS B2 (1.8 OZ/SQ. FT.) SIZED AS REQUIRED TO PROJECT 3" INTO MASONRY. SPACE ANCHORS AT 16" O/C.

O. PROVIDE CONTROL JOINTS AT 20' MAXIMUM ON CENTER IN MASONRY WALLS.

P. LAP SPLICES IN REINFORCING 48 BAR DIAMETER MINIMUM. UNLESS NOTED OTHERWISE, VERTICAL REINFORCING TO BE FULL HEIGHT OF WALL AND DOWELED INTO FOOTINGS.

Q. SHORE REINFORCED WALLS UNTIL CONCRETE SLABS ARE POURED AND CURED.

R. SUBMIT MATERIAL CERTIFICATION FOR:
1. BLOCK UNITS
2. MORTAR
3. GROUT
4. HORIZONTAL JOINT REINFORCING

4.4 STEEL & PRECAST LINTEL SCHEDULE

A. PROVIDE AND INSTALL LINTELS FOR OPENINGS IN MASONRY WALLS (NOT TO BE LIMITED TO OPENINGS SHOWN ON STRUCTURAL PLANS). UTILIZE LINTEL SIZES AS INDICATED ON THE SCHEDULE BELOW, UNLESS NOTED OTHERWISE ON PLAN. (COORDINATE OPENINGS FOR MECHANICAL TRADES, ARCHITECTURAL OPENINGS IN NON BEARING WALLS, ETC.)

B. WELD MULTIPLE ANGLE LINTELS AT ENDS AND 1/3 POINTS OF SPAN.

C. SHORE LINTELS TO PREVENT ROTATION DURING CONSTRUCTION.

D. LINTELS TO HAVE MINIMUM 8" BEARING ON SOLID MASONRY FOR A MINIMUM 16" DEEP EACH END, UNLESS NOTED OTHERWISE.

MARK	MATERIALS	REMARKS
L-1	1-L4x3½x5/16 LLV FOR EACH 4" WALL THICKNESS FOR OPENINGS UP TO 6'-0"	FOR CAVITY WALLS, REPLACE 1-L4x3½x5/16 LLV WITH 1-L5x5x5/16
L-2	1-L6x3½x5/16 LLV FOR EACH 4" WALL THICKNESS FOR OPENINGS UP TO 6'-1" TO 10'-0"	FOR CAVITY WALLS, REPLACE 1-L6x3½x5/16 LLV WITH 1-L5x5x5/16
P-1	1-4x8 PRECAST MASONRY LINTEL EACH 4" WALL THICKNESS OR 1-6x8 EACH 6" WALL THICKNESS WITH 1-#4 BOTTOM BAR	FOR OPENINGS UP TO 2'-8"
P-2	1-4X8 PRECAST MASONRY LINTEL EACH 4" WALL THICKNESS OR 1-6x8 EACH 6" WALL THICKNESS WITH 1-#3 TOP AND 1-#4 BOTTOM BAR	FOR OPENINGS UP TO 3'-0" TO 6'-0"
P-3	1-4X8 PRECAST MASONRY LINTEL EACH 4" WALL THICKNESS OR 1-6x8 EACH 6" WALL THICKNESS WITH 1-#3 TOP AND 1-#5 BOTTOM BAR	FOR OPENINGS UP TO 6'-1" TO 10'-0"

5.1 STRUCTURAL STEEL

A. UNLESS GOVERNED BY BUILDING CODE OR LOCAL AMENDMENTS: FABRICATE AND ERECT STRUCTURAL STEEL IN ACCORDANCE WITH AISC MANUAL OF STEEL CONSTRUCTION, THIRTEENTH EDITION AND OSHA STEEL ERECTION STANDARDS UNLESS NOTED ON DRAWINGS OR SPECIFICATIONS. WHEN THERE IS A CONFLICT, THE MOST STRINGENT IS TO APPLY.

B. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION. REPRINTS OF CONTRACT DOCUMENTS ARE NOT ACCEPTABLE.

C. STEEL - ASTM A-36 FOR ANGLES, CHANNELS, AND MISCELLANEOUS SHAPES.
- ASTM A-992 (50 KSI) FOR WF SHAPES.
STEEL TO BE OF AMERICAN ORIGIN ONLY.

D. STRUCTURAL TUBES - ASTM A-500 (GRADE B).

E. STRUCTURAL PIPES - ASTM A-501, OR ASTM A-53, TYPE E, GRADE B.

F. SUPPLY STEEL LINTELS REQUIRED FOR WALL SUPPORT. LINTELS WILL BE INSTALLED UNDER MASONRY DIVISION.

G. COLUMN BASE ANCHOR RODS - ASTM F-1554, GRADE 36, 55, 105. HOOKED, HEADED, OR THREADED ANCHOR RODS - ASTM A-307, GRADE A.

H. NUTS - ASTM A-563, HEAVY.

I. WASHERS - ASTM F-436.

J. PLATE WASHERS - ASTM A-36.

K. HIGH STRENGTH BOLTS FOR CONNECTIONS - ASTM A-325 OR A-490.

L. GROUT FOR UNDER BASE AND BEARING PLATES: ASTM C-1107, 5000 PSI.

M. WELDERS SHALL BE CERTIFIED IN ACCORDANCE WITH THE A.W.S.

N. COORDINATE WELDING ELECTRODES, MACHINES, ETC., WITH TYPE OF STEEL BEING WELDED.

O. GUY AND BRACE STEEL FRAME TO MAINTAIN STABILITY OF BUILDING.

P. PROVIDE 1/2" DIA. BOLT AND WASHER AND 1 1/4" ROUND HOLE FOR CLIP AND CONTINUOUS ANGLES ATTACHED TO STEEL BEAMS OR COLUMNS.

Q. ENDS OF BEAMS BEARING ON TOP OF COLUMNS TO HAVE 1/4" STIFFENER PLATES EACH SIDE (SEE DETAILS).

R. PROVIDE 1/2" DIA. ERECTION BOLTS FOR CLIP AND CONTINUOUS ANGLES.

S. AFTER ADJUSTMENT AND FINISHED ALIGNMENT, PROVIDE 2" OF 3/16" FILLET WELD AT EACH BOLT LOCATION, UNLESS OTHERWISE NOTED.

T. COAT STEEL EXPOSED AFTER BUILDING IS COMPLETED WITH ONE SHOP COAT OF AN APPROVED RUST INHIBITIVE PRIMER. PAINT STEEL EXPOSED TO WEATHER AFTER BUILDING IS COMPLETED WITH TWO ADDITIONAL COATS OF RUST INHIBITIVE PAINT AFTER ERECTION. PAINT SHALL BE COMPATIBLE WITH SHOP COAT.

U. ENGAGE THE SERVICES OF AN QUALIFIED INSPECTION AND TESTING AGENCY TO INSPECT STRUCTURAL STEEL PLACEMENT AND CONNECTIONS.

5.2 STEEL JOISTS

A. MANUFACTURE AND INSTALL STEEL JOISTS AND BRIDGING (WHEN NOT GOVERNED BY CODE REQUIREMENTS) IN ACCORDANCE WITH THE "STANDARD SPECIFICATION, LOAD TABLES, AND WEIGHT TABLES FOR STEEL JOISTS AND JOIST GIRDERS" AND OSHA STEEL ERECTION STANDARDS, UNLESS NOTED ON DRAWINGS OR SPECIFICATIONS.

B. BRIDGING FOR K- AND LH- SERIES JOISTS TO BE HORIZONTAL FOR SPANS UP TO AND INCLUDING 60FT. EXCEPT WHERE CODE REQUIRES FOR ERECTION STABILITY AND/OR THE STEEL JOIST INSTITUTE SPECIFICATIONS REQUIRE BOLTED DIAGONAL BRIDGING. LH- AND DLH- SERIES JOISTS EXCEEDING 60 FT. IN LENGTH SHALL HAVE BOLTED DIAGONAL BRIDGING FOR ALL ROWS.

C. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION. REPRINTS OF CONTRACT DOCUMENTS ARE NOT ACCEPTABLE.

D. WELDERS IN FIELD SHALL BE CERTIFIED IN ACCORDANCE WITH AWS.

E. PROVIDE BEARING PLATES FOR JOISTS BEARING ON MASONRY WALLS (SEE DETAIL).

F. STEEL JOISTS (ROOF AND FLOOR) TO BE ANCHORED TO MASONRY WALLS (SEE DETAIL).

G. GROUT FOR UNDER JOIST BEARING PLATES: ASTM C-1107, 5000 PSI.

H. FOR BEAMS HAVING JOISTS BEARING ON ONE SIDE ONLY, EXTEND JOIST SEAT 1/2" MIN. BEYOND BEAM CENTER LINE.

I. WHEN BEAM FLANGE IS NOT AT LEAST TWICE THE MINIMUM JOIST BEARING DIMENSION, STAGGER JOISTS.

J. WHEN BEAM FLANGE IS 5" OR LARGER, AND JOISTS BEAR FROM EACH SIDE, EXTEND JOIST TO WITHIN 1/8" OF CENTER OF BEAM ON EACH SIDE.

K. ENGAGE THE SERVICES OF AN INDEPENDENT INSPECTION AGENCY TO PERFORM FIELD INSPECTIONS OF STEEL JOISTS AND ACCESSORIES.

5.5 METAL DECK

A. DESIGN AND MANUFACTURE METAL DECK IN CONFORMANCE WITH THE "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS" AS PUBLISHED BY AISI.

B. SUBMIT COMPLETE SHOP AND ERECTION DRAWINGS FOR REVIEW PRIOR TO FABRICATION OR ERECTION. REPRINTS OF CONTRACT DOCUMENTS ARE NOT ACCEPTABLE.

C. CONNECT DECK TO SUPPORTING STEEL MEMBERS AS FOLLOWS (UNLESS OTHERWISE NOTED ON DRAWINGS): INTERMEDIATE BEARING - 5/8" PUDDLE WELDS AT 18" O/C: PERIMETER END BEARING - 5/8" PUDDLE WELDS AT 12" O/C: SIDE LAPS - #10 SELF TAPPING SCREWS AT 20" O/C: PERIMETER EDGE - 5/8" PUDDLE WELDS AT 20" O/C.

D. ENGAGE THE SERVICES OF AN INDEPENDENT AGENCY TO FIELD INSPECT THE METAL DECK AND ACCESSORIES.

5.6 COLD FORMED METAL FRAMING

A. DESIGN AND INSTALL COLD FORMED METAL FRAMING IN COMPLIANCE WITH THE "SPECIFICATIONS FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS," LATEST EDITION, AS PUBLISHED BY AISI.

B. SUBMIT TO THE ARCHITECT FOR REVIEW, PRIOR TO FABRICATION, COMPLETE SHOP DRAWINGS OF LIGHT GAGE FRAMING ELEMENTS. SHOP DRAWINGS SHALL INCLUDE COMPLETE SECTION PROPERTIES OF MEMBERS, CONNECTION DETAILS, BRIDGING SIZE, LOCATION AND ERECTION PLANS, AND STRUCTURAL ANALYSIS DATA SIGNED AND SEALED BY THE QUALIFIED PROFESSIONAL ENGINEER RESPONSIBLE FOR THEIR PREPARATION. REPRINTS OF CONTRACT DRAWINGS ARE NOT ACCEPTABLE.

C. PROVIDE A CONTINUOUS STEEL TRACK AT THE TOP AND BOTTOM OF STUD WALLS. HORIZONTAL BRIDGING FOR WALLS UP TO 10'- 0" HIGH TO BE 2 ROWS AND FOR WALL OVER 10'- 0" HIGH TO BE SPACED MAX. 4'- 0" O/C. CONNECT BRIDGING TO EACH FLANGE ON EACH STUD. DETAILS SHALL APPEAR ON SHOP DRAWINGS.

D. COLD FORMED METAL FRAMING FOR EXTERIOR WALLS, INTERIOR BEARING WALLS, AND JOISTS SHALL NOT BE LESS THAN 18 GAGE.

E. COLD FORMED METAL FRAMING SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A-653. MINIMUM G90 COATING FOR MEMBERS IN EXTERIOR WALL CONSTRUCTION AND G60 FOR INTERIOR WALL FRAMING MEMBERS.

F. UNLESS NOTED OTHERWISE ON SHOP DRAWINGS, SCREW CONNECTIONS OF STUD-TO-STUD, STUD-TO-TRACK, AND STUD-TO-FRAME USING 2-NO. 8 SELF TAPPING METAL SCREWS AT EACH CONNECTION.

G. PROVIDE DOUBLE JAMB EACH END OF LINTEL, UNLESS NOTED OTHERWISE ON PLAN OR SHOP DRAWINGS.

H. CONNECT STUD FLANGES TO TRACK. STUDS SHALL HAVE FULL BEARING AGAINST INSIDE WEB OF TRACK. NO VOIDS WILL BE PERMITTED AT TOP OR BOTTOM OF STUD AT TRACK (TYPICAL).

I. PROVIDE VERTICAL STUD UNDER EACH JOIST (TYPICAL).

J. WELD CONNECTIONS OF STUD-TO-STUD, STUD-TO-TRACK, AND STUD-TO-FRAME USING 2-1/8" FILLET WELDS 1 1/2" LONG, UNLESS OTHERWISE NOTED ON THE DRAWINGS. WELDING TO BE PERFORMED IN ACCORDANCE WITH AWS D.1.3, 1981 "STRUCTURAL WELDING CODE-SHEET STEEL." TOUCH UP WELDS WITH ZINC RICH PRIMER AFTER WELDS HAVE BEEN INSPECTED AND APPROVED. (ALT) SCREW CONNECTIONS OF STUD-TO-STUD, STUD-TO-TRACK, AND STUD-TO-FRAME USING 2-NO. 8 SELF TAPPING METAL SCREWS AT EACH CONNECTION.

K. SPLICES IN STRUCTURAL MEMBERS WILL NOT BE PERMITTED.

L. JOIST BRIDGING TO BE 16 GAGE SOLID. CONNECT BRIDGING TO JOISTS WITH 2 - NO. 8 METAL SCREWS TOP AND BOTTOM. SPACE BRIDGING AS FOLLOWS:
UP TO 14' 1 ROW
14' TO 20' 2 ROWS
20' TO 26' 3 ROWS

M. ENGAGE THE SERVICES OF AN INDEPENDENT AGENCY TO FIELD INSPECT THE COLD FORMED METAL FRAMING AND COMPONENTS.

5.6B COLD FORMED METAL LINTEL SCHEDULE

A. PROVIDE AND INSTALL LINTELS FOR OPENINGS IN EXTERIOR WALLS AND INTERIOR BEARING WALLS.

B. OPENINGS FOR OTHER TRADES MAY NOT BE INDICATED ON THE STRUCTURAL DRAWINGS. COORDINATE OPENINGS FOR MECHANICAL TRADES, ARCHITECTURAL OPENINGS IN EXTERIOR WALLS AND INTERIOR BEARING WALLS, ETC. LINTELS FOR OPENINGS IN INTERIOR NON-BEARING WALLS PER MANUFACTURER'S DESIGN AND RECOMMENDATIONS.

C. UTILIZE MINIMUM LINTEL SIZES AS INDICATED ON THE SCHEDULE BELOW, UNLESS NOTED OTHERWISE ON PLAN OR PER MANUFACTURER'S RECOMMENDATIONS, WHICH EVER IS GREATER.

D. SHOP DRAWINGS TO INDICATE CONNECTIONS OF LINTELS TO JAMBS.

E. LINTELS TO HAVE MINIMUM 16 GAUGE TOP AND BOTTOM TRACK UNLESS NOTED OTHERWISE ON SHOP DRAWINGS.

LIGHT GAUGE LINTEL SCHEDULE

MARK	MATERIALS	REMARKS
SL-1	(2) - 4" X 1 5/8" X 16 GAUGE	FOR OPENINGS UP TO 3'-6", FOR 4" WALLS
SL-2	(2) - 6" X 1 5/8" X 16 GAUGE	FOR OPENINGS 3'-7" UP TO 5'-3", FOR 4" OR 6" WALLS
SL-3	(3) - 6" X 1 5/8" X 16 GAUGE	FOR OPENINGS 5'-4" UP TO 6'-6", FOR 6" WALLS
SL-4	(3) - 8" X 1 5/8" X 16 GAUGE	FOR OPENINGS 6'-7" UP TO 7'-6", FOR 6" WALLS

NOTE: ALL LINTELS TO HAVE 6" X 16 GA. TOP AND BOTTOM TRACKS

L2M
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Cromwell Business Park at BWI
Glen Burnie, Maryland 21061
L2M@L2M.com
Phone 410.863.1302
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I certify that these documents were prepared or approved by me, and that I am a duly licensed professional engineer under the laws of the state of Maryland License# 200363 Expires: 19 March 2019.

DASH IN
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